



FINITE ELEMENT METHOD Brick elements

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Introduction

Brick is the Three-dimensional element, which can be defined as a body, to which all dimensions are of the same order. The shape of the body and the load is any of available. With brick elements, fully 3D solid constructions can be modeled.

Introduction

Brick elements can replace every other type of element, like frame, shell and plate elements. A solid model is divided into brick elements and the geometric shape of the elements can be tetrahedron, hexahedron or prism with triangle base, which means, that a brick element is build of triangles and quadrilateral



Three-dimensional shapes of brick elements







six-nodes



eight-nodes (hexahedron)



We will show how the tetrahedron is regarded in Finite Element Method. You can see how displacements and nodal forces are located



Introduction

As it can be seen, in each node there are three displacements and forces in all global directions, but there are no rotations and moments. Vector of movements of element nodes and nodal forces can be written as follows



Vector of movements of element nodes and nodal forces $\begin{bmatrix} u_{ix} \end{bmatrix} \begin{bmatrix} F_{ix} \end{bmatrix}$

$$\mathbf{u}^{\prime \mathbf{e}} = \begin{vmatrix} u_{ix} \\ u_{iy} \\ u_{iz} \\ u_{jx} \\ u_{jx} \\ u_{jy} \\ u_{jy} \\ u_{jz} \\ u_{kx} \\ u_{kx} \\ u_{ky} \\ u_{kz} \\ u_{lx} \\ u_{lx} \\ u_{lx} \\ u_{ly} \\ u_{lz} \end{vmatrix} \qquad \mathbf{f}^{\prime \mathbf{e}} = \begin{vmatrix} F_{ix} \\ F_{jy} \\ F_{jz} \\ F_{kx} \\ F_{ky} \\ F_{kz} \\ F_{lx} \\ F_{ly} \\ F_{lz} \end{vmatrix}$$

The brick works in a spatial state of stress

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In brick element, there are three components of normal stresses σ_{xx} , σ_{yy} , σ_{zz} , and six sheer stresses, but according to the symmetry of a stress tensor components of shear stress we have $\tau_{xy}=\tau_{yx}$, $\tau_{xz}=\tau_{zx}$, and $\tau_{zy}=\tau_{yz}$, thus we have six independent components of stress which are composed in the stress vector

components of the stress vector

$$\boldsymbol{\sigma} = \begin{bmatrix} \boldsymbol{\sigma}_{x} \\ \boldsymbol{\sigma}_{y} \\ \boldsymbol{\sigma}_{z} \\ \boldsymbol{\tau}_{xy} \\ \boldsymbol{\tau}_{yz} \\ \boldsymbol{\tau}_{zx} \end{bmatrix}$$

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Because of the fact that the brick works in threedimensional state of stress and strain, the strain vector is similar to stress vector. Relationships between the components of displacement and strain vectors are similar to those for 2D elements

$$\boldsymbol{\varepsilon} = \begin{bmatrix} \varepsilon_{x} \\ \varepsilon_{y} \\ \varepsilon_{z} \\ \gamma_{xy} \\ \gamma_{yz} \\ \gamma_{zx} \end{bmatrix} = \begin{bmatrix} \frac{\partial u_{x}}{\partial x} \\ \frac{\partial u_{y}}{\partial y} \\ \frac{\partial u_{z}}{\partial z} \\ \frac{\partial u_{x}}{\partial y} + \frac{\partial u_{y}}{\partial x} \\ \frac{\partial u_{y}}{\partial z} + \frac{\partial u_{z}}{\partial y} \\ \frac{\partial u_{z}}{\partial z} + \frac{\partial u_{z}}{\partial z} \end{bmatrix}$$

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The relationship between vectors **σ** and **ε** is, like in two-dimensional elements, described by constitutive equations. For elastic isotropic materials the constitutive equation is shown below

$\boldsymbol{\sigma} = \mathbf{D} \boldsymbol{\cdot} \boldsymbol{\varepsilon}$

where **D** is the square matrix with dimensions 6x6 containing the material constants

Stiffness matrix of 3D element

The stiffness matrix is the same as for all previous chapters

 $\mathbf{f}^{e} = \mathbf{K}^{e} \mathbf{u}^{e}$

where \mathbf{K}^{e} has the same formula as in previous chapters

Stiffness matrix of 3D eeagrants

$$\mathbf{K}^{e} = \int_{V} \left(\mathbf{B}^{e} \right)^{\mathrm{T}} \mathbf{D} \, \mathbf{B}^{e} dV$$

In case of brick element \mathbf{B}^e has following formula $\mathbf{B}^e = \Re \mathbf{N}$

where N is matrix of shape function (which will be described later), \Re is a matrix of differential operators

Stiffness matrix of 3D element

$$\Re = \begin{bmatrix} \frac{\partial}{\partial x} & 0 & 0 \\ 0 & \frac{\partial}{\partial y} & 0 \\ 0 & 0 & \frac{\partial}{\partial z} \\ \frac{\partial}{\partial y} & \frac{\partial}{\partial z} & 0 \\ \frac{\partial}{\partial y} & \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial z} & \frac{\partial}{\partial y} \\ \frac{\partial}{\partial z} & 0 & \frac{\partial}{\partial x} \end{bmatrix}$$

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Shape function of 3D eea grants Signature of the second se

Displacement of every point of brick element in three-dimensional coordinate system can be written as a function

$$u_x(x, y, z) = a_1 + a_2 x + a_3 y + a_4 z$$

$$u_y(x, y, z) = b_1 + b_2 x + b_3 y + b_4 z$$

$$u_{z}(x, y, z) = c_{1} + c_{2}x + c_{3}y + c_{4}z$$



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Equation
$$u_x(x, y, z) = a_1 + a_2 x + a_3 y + a_4 z$$

 $u_y(x, y, z) = b_1 + b_2 x + b_3 y + b_4 z$
 $u_z(x, y, z) = c_1 + c_2 x + c_3 y + c_4 z$

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can be written as a function depending on displacements in every of four nodes in tetrahedral element

$$u_{x}(x, y, z) = N_{i}(x, y, z)u_{ix} + N_{j}(x, y, z)u_{jx} + N_{k}(x, y, z)u_{kx} + N_{l}(x, y, z)u_{lx}$$
$$u_{y}(x, y, z) = N_{i}(x, y, z)u_{iy} + N_{j}(x, y, z)u_{jy} + N_{k}(x, y, z)u_{ky} + N_{l}(x, y, z)u_{ly}$$
$$u_{z}(x, y, z) = N_{i}(x, y, z)u_{iz} + N_{j}(x, y, z)u_{jz} + N_{k}(x, y, z)u_{kz} + N_{l}(x, y, z)u_{lz}$$
where

$$N_i(x, y, z) = a_i + b_i x + c_i y + d_i z$$

$$N_j(x, y, z) = a_j + b_j x + c_j y + d_j z$$

$$N_k(x, y, z) = a_k + b_k x + c_k y + d_k z$$

$$N_l(x, y, z) = a_l + b_l x + c_l y + d_l z$$

And this equation can be written in matrix form

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 $\mathbf{u}(x, y, z) = \mathbf{N}\mathbf{u}'^e$

where $\mathbf{u}(x, y, z)$ is a displacement vector of any point located inside the brick element, \mathbf{u}'^e is nodal displacement vector, and \mathbf{N} is stiffness function matrix

$$\mathbf{u}(x, y, z) = \begin{bmatrix} u_x(x, y, z) \\ u_y(x, y, z) \\ u_z(x, y, z) \end{bmatrix}$$

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$$\mathbf{N} = \begin{bmatrix} N_i & 0 & 0 & N_j & 0 & 0 & N_k & 0 & 0 & N_l & 0 & 0 \\ 0 & N_i & 0 & 0 & N_j & 0 & 0 & N_k & 0 & 0 & N_l & 0 \\ 0 & 0 & N_i & 0 & 0 & N_j & 0 & 0 & N_k & 0 & 0 & N_l \end{bmatrix}$$

Shape function of 3D eeagrants Springer grants

Shape function for element of eight nodes can be written for any node as bellow

$$N_i = \frac{1}{8}(1 + \xi_o)(1 + \eta_o)(1 + \zeta_o)$$
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where

$$\xi_o = \pm \frac{x}{l_x} \qquad \qquad \eta_o = \pm \frac{y}{l_y} \qquad \qquad \zeta_o = \pm \frac{z}{l_z}$$

This shape function is based on Lagrangian interpolation for the three variables of function passing through two points. Designations for eight-node element are shown in next Figure

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$$\mathbf{B}^{e} = \Re \mathbf{N} = \begin{bmatrix} \frac{\partial N_{i}}{\partial x} & 0 & 0 & \frac{\partial N_{j}}{\partial x} & 0 & 0 & \frac{\partial N_{k}}{\partial x} & 0 & 0 & \frac{\partial N_{l}}{\partial x} & 0 & 0 \\ 0 & \frac{\partial N_{i}}{\partial y} & 0 & 0 & \frac{\partial N_{j}}{\partial y} & 0 & 0 & \frac{\partial N_{k}}{\partial y} & 0 & 0 & \frac{\partial N_{l}}{\partial y} & 0 \\ 0 & 0 & \frac{\partial N_{i}}{\partial z} & 0 & 0 & \frac{\partial N_{j}}{\partial z} & 0 & 0 & \frac{\partial N_{k}}{\partial z} & 0 & 0 & \frac{\partial N_{l}}{\partial z} \\ \frac{\partial N_{i}}{\partial y} & \frac{\partial N_{i}}{\partial x} & 0 & \frac{\partial N_{j}}{\partial y} & \frac{\partial N_{j}}{\partial x} & 0 & \frac{\partial N_{k}}{\partial y} & \frac{\partial N_{k}}{\partial x} & 0 & \frac{\partial N_{l}}{\partial y} & \frac{\partial N_{l}}{\partial z} & 0 \\ 0 & \frac{\partial N_{i}}{\partial z} & \frac{\partial N_{i}}{\partial y} & 0 & \frac{\partial N_{j}}{\partial z} & \frac{\partial N_{j}}{\partial y} & 0 & \frac{\partial N_{k}}{\partial z} & \frac{\partial N_{k}}{\partial y} & 0 & \frac{\partial N_{k}}{\partial z} & \frac{\partial N_{k}}{\partial y} & 0 & \frac{\partial N_{l}}{\partial z} & \frac{\partial N_{l}}{\partial y} \\ \frac{\partial N_{i}}{\partial z} & 0 & \frac{\partial N_{i}}{\partial y} & \frac{\partial N_{j}}{\partial z} & 0 & \frac{\partial N_{j}}{\partial x} & \frac{\partial N_{k}}{\partial z} & 0 & \frac{\partial N_{k}}{\partial z} & \frac{\partial N_{l}}{\partial z} & 0 & \frac{\partial N_{l}}{\partial z} \end{bmatrix}$$

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we can obtain in simplified form

$$\mathbf{B}^{e} = \begin{bmatrix} b_{i} & 0 & 0 & b_{j} & 0 & 0 & b_{k} & 0 & 0 & b_{l} & 0 & 0 \\ 0 & c_{i} & 0 & 0 & c_{j} & 0 & 0 & c_{k} & 0 & 0 & c_{l} & 0 \\ 0 & 0 & d_{i} & 0 & 0 & d_{j} & 0 & 0 & d_{k} & 0 & 0 & d_{l} \\ c_{i} & b_{i} & 0 & c_{j} & b_{j} & 0 & c_{k} & b_{k} & 0 & c_{l} & b_{l} & 0 \\ 0 & d_{i} & c_{i} & 0 & d_{j} & c_{j} & 0 & d_{k} & c_{k} & 0 & d_{l} & c_{l} \\ d_{i} & 0 & b_{i} & d_{j} & 0 & b_{j} & d_{k} & 0 & b_{k} & d_{l} & 0 & b_{l} \end{bmatrix}$$



Now after knowing the geometric matrix for tetrahedral element, the strain vector can be obtained from the following equation

$$\boldsymbol{\varepsilon} = \Re \mathbf{N}(x, y, z) \mathbf{u}^{\prime e} = \mathbf{B}^{e} \mathbf{u}^{\prime e}$$

which is

element on tetrahedron eea norway grants grants example \mathcal{U}_{ix} \mathcal{U}_{iv} \mathcal{U}_{iz} $\begin{bmatrix} \varepsilon_{x} \\ \varepsilon_{y} \\ \varepsilon_{z} \\ \gamma_{xy} \\ \gamma_{yz} \\ \gamma_{zx} \end{bmatrix} = \begin{bmatrix} b_{i} & 0 & 0 & b_{j} & 0 & 0 & b_{k} & 0 & 0 & b_{l} & 0 \\ 0 & c_{i} & 0 & 0 & c_{j} & 0 & 0 & c_{k} & 0 & 0 & c_{l} \\ 0 & 0 & d_{i} & 0 & 0 & d_{j} & 0 & 0 & d_{k} & 0 & 0 \\ c_{i} & b_{i} & 0 & c_{j} & b_{j} & 0 & c_{k} & b_{k} & 0 & c_{l} & b_{l} \\ 0 & d_{i} & c_{i} & 0 & d_{j} & c_{j} & 0 & d_{k} & c_{k} & 0 & d_{l} \\ d_{i} & 0 & b_{i} & d_{j} & 0 & b_{j} & d_{k} & 0 & b_{k} & d_{l} & 0 \end{bmatrix}$ \mathcal{U}_{jx} \mathcal{U}_{jy} d_1 \mathcal{U}_{jz} 0 \mathcal{U}_{kx} C_l \mathcal{U}_{ky} b_l \mathcal{U}_{kz} u_{lx} u_{lv}

 \mathcal{U}_{lz}

element on tetrahedron example

Knowing the strain vector it can be used in equation $\sigma = \mathbf{D} \cdot \mathbf{\epsilon}$ to obtain the stress vector



Example of the3D brick elements use

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- There are some rules that are need to be followed:
- FEM grid must take into account the shape of the structure,
- in a hole in the model, nodes must be located so that there is no possibility to create a FEM element, but to form an empty area,
- mesh nodes must be located in location of concentrated loads,

mesh nodes must be located in the boundary condition points,

- the edges of the grid must be located on the border between parts of elements made of different materials,
- if the job is symmetrical, the mesh also should be symmetrical

- FEM mesh should be concentrated in areas of high stress concentration and in areas of rapid change in stress (high value of gradient). Such areas are located:
- at the corners,
- around the points of application of concentrated forces,
- around the supports,

If the component is narrow then in cross section there should be at least four belts of elements. Only then they will be able to describe the change in stress in the cross section.